DRAFT ENVIRONMENTAL IMPACT STATEMENT

408 PERMISSION AND 404 PERMIT TO THREE RIVERS LEVEE IMPROVEMENT AUTHORITY

FOR THE

FEATHER RIVER LEVEE REPAIR PROJECT, CALIFORNIA SEGMENT 2

APPENDIX H – TECHNICAL MEMORANDUM: CUMULATIVE HYDRAULIC IMPACTS OF ADDING SUTTER COUNTY'S PROPOSED FEATHER RIVER WEST LEVEE SETBACK NEAR STAR BEND TO TRLIA'S FEATHER RIVER EAST LEVEE SETBACK

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Technical Memorandum

DATE: December 12, 2007

TO: Ric Reinhardt

FROM: Ben Tustison

SUBJECT: Cumulative Hydraulic Impacts of Adding Sutter County's Proposed

Feather River West Levee Setback near Star Bend to TRLIA's Feather

River East Levee Setback

Purpose

The purpose of this memorandum is to report the computed cumulative hydraulic impacts of Sutter County's proposed setback, the Feather River west levee near Star Bend (between Lower Feather River Floodplain Mapping Study River Miles 16.5 and 18.25), and the TRLIA Feather River east levee setback (River Miles 17.0 to 24.3). Figure 1 shows a plan view of the Star Bend area with the existing and proposed Feather River west levee alignments and the locations of the model cross-sections affected by the setback. The TRLIA Feather River east levee setback is also shown on this figure. Figure 2 shows the profile views of these eight cross-sections. The distance which the west levee is set back for each section is specified on these plots.

Analysis

This analysis relied on the hydraulic model developed by the U.S. Army Corps of Engineers for the report entitled "Amended Draft Lower Feather River Floodplain Mapping Study – Revised February 17, 2005". This model was developed using the software HEC-RAS, developed by the Corps' Hydrologic Engineering Center. The model has been revised by MBK Engineers. MBK Version 9 of this model was utilized for this analysis. The details of this model can be found in the MBK report "Hydraulic and Hydrologic Documentation for FEMA Certification of the Three River's Levee Improvement Authority Project" from March 2007.

Assumptions

Levee Performance

An important assumption in performing hydraulic simulations of leveed systems on a regional basis is defining if, when, and how levee failures will occur. The analysis as presented herein assumes that levees would not fail before or after overtopping. Top-of-levee profiles were compared to calculated 1-in-200 water surface profiles to determine low spots where levee overtopping may occur. The locations were defined in the HEC-RAS model.

Storm Centering

The Feather River at Shanghai with Yuba River emphasis storm centering was used for this analysis.

Unsteady Flow Analysis

The 1-in-2, 1-in-10, 1-in-25, 1-in-50, 1-in-100 and 1-in-200 annual exceedence probability (AEP) water surfaces were evaluated for the unsteady flow analysis.

Steady Flow Analysis

For each reach in the hydraulic model with a specified 1957 Design Flood Plane flow rate, the respective 1957 Design Flood Plane flow rates were used. 300,000 cfs is the specified 1957 Design Flood Plane flow rate for the project reach (Feather River between Yuba River and Bear River). At the downstream boundaries of the model on the Sacramento River and Yolo Bypass, the specified 1957 Design Flood Plane water surface elevations were adopted for this analysis.

Scenarios

Two scenarios were evaluated for this analysis: an Existing conditions scenario and a Project conditions scenario.

Existing Conditions

This represents the existing condition (December 2006) of the flood control system of the Yuba-Feather-Bear Rivers and their tributaries. Channel and levee conditions were represented with the best estimate of present conditions. This scenario also contains the proposed Feather River Setback Levee Project, which consists of setting back the Feather River east levee from River Mile 17.0 to 24.3 and habitat enhancement in the setback levee area.

Project Conditions

This represents the existing conditions scenario (TRLIA setback) <u>plus</u> Sutter County's proposed set back of the Feather River west levee from River Mile 16.5 to 18.25. The setback configuration and hydraulic parameters for model cross-sections 16.5 to 18.25 were provided by Wood Rodgers. For cross-sections 17.0 to 18.25, roughness values for the channel and overbank of 0.039 and 0.088, respectively, were adopted as opposed to the values of 0.035 and 0.08 provided by Wood Rodgers. This was done to retain consistency with the roughness characteristics of the remainder of the Feather River east levee setback, represented in the cross-sections between 18.25 and 24.25.

Summary

Table 1 shows the maximum water surface elevations for the Existing and Project conditions, respectively, within and surrounding the project reach for the 1-in-2 AEP through 1-in-200 AEP flood events and the 1957 Design Flood Plane flow analysis.

Figure 3 shows the maximum water surface profiles for the Existing and Project conditions, respectively, within and surrounding the project reach.

For the 1-in-100 AEP and 1-in-200 AEP flood events, the maximum water surface additional reductions due to the Project (Star Bend Setback Levee) are 0.21 feet and 0.25 feet, respectively. Both of these maximum additional reductions occur at River Mile 18.25. The maximum water surface increases due to the Project for the 1-in-100 AEP and 1-in-200 AEP flood events are 0.02 feet and 0.04 feet, respectively, both occurring at River Mile 16.50. There are no measurable water surface changes due to the Project below River Mile 15.0.

For the 1957 Design Flood Plane steady state flow analysis, the maximum additional water surface reduction due to the Project (Star Bend Setback levee) is 0.24 feet at River Mile 18.25. The maximum water surface increase due to the Project is 0.05 feet at River Mile 16.75. There are no measurable water surface changes due to the Project below River Mile 15.0.

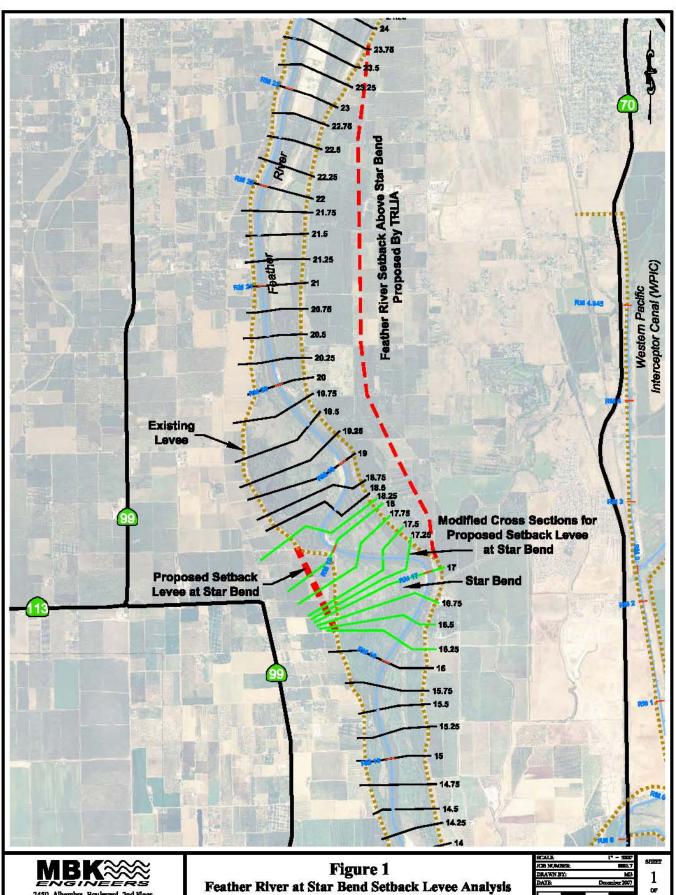
Sincerely,

Ben Tustison, P.E.

Star Bend SB Tech Memo 12 12 2007.doc

Table 1. Existing and Proposed Feather River Water Surface Elevations, River Miles 15.0 to 23.0.

Lower Feather	Unsteady Flow													Steady Flow	
River Floodplain Mapping Study	1-in-2 AEP		1-in-10 AEP		1-in-25 AEP		1-in-50 AEP		1-in-100 AEP		1-in-200 AEP		1957 Design Flow		
River Mile	Existing	Proposed	Existing	Proposed	Existing	Proposed	Existing	Proposed	Existing	Proposed	Existing	Proposed	Existing	Proposed	
23.00	51.14	51.09	60.25	60.16	63.44	63.34	64.73	64.63	65.03	64.92	68.25	68.12	66.13	66.01	
22.75	50.78	50.72	59.93	59.84	63.13	63.02	64.42	64.31	64.71	64.60	67.93	67.79	65.84	65.72	
22.50	50.25	50.19	59.45	59.35	62.66	62.55	63.95	63.83	64.25	64.13	67.47	67.32	65.32	65.19	
22.25	49.93	49.87	59.05	58.94	62.26	62.14	63.55	63.43	63.86	63.73	67.08	66.92	64.94	64.80	
22.00	49.62	49.56	58.77	58.66	61.98	61.86	63.28	63.14	63.58	63.45	66.80	66.63	64.71	64.56	
21.75	49.21	49.14	58.34	58.21	61.58	61.44	62.88	62.74	63.19	63.04	66.42	66.25	64.25	64.09	
21.50	48.84	48.76	57.98	57.84	61.24	61.09	62.55	62.40	62.86	62.71	66.09	65.91	63.97	63.80	
21.25	48.41	48.32	57.53	57.39	60.83	60.67	62.15	61.99	62.46	62.30	65.72	65.53	63.57	63.39	
21.00	48.18	48.09	57.23	57.07	60.53	60.36	61.85	61.68	62.17	62.00	65.42	65.22	63.29	63.10	
20.75	47.97	47.88	57.01	56.84	60.30	60.13	61.62	61.45	61.94	61.77	65.19	64.99	63.09	62.89	
20.50	47.84	47.74	56.84	56.67	60.12	59.94	61.43	61.26	61.76	61.58	64.99	64.78	62.91	62.71	
20.25	47.69	47.59	56.65	56.48	59.93	59.75	61.24	61.06	61.57	61.39	64.79	64.57	62.72	62.52	
20.00	47.53	47.43	56.48	56.30	59.75	59.57	61.06	60.87	61.39	61.20	64.60	64.38	62.56	62.35	
19.75	47.37	47.26	56.30	56.12	59.57	59.38	60.88	60.69	61.21	61.02	64.42	64.20	62.39	62.17	
19.50	47.23	47.12	56.15	55.96	59.43	59.23	60.74	60.55	61.08	60.88	64.29	64.06	62.26	62.04	
19.25	47.13	47.02	56.03	55.84	59.31	59.11	60.63	60.43	60.96	60.76	64.17	63.94	62.15	61.93	
19.00	47.01	46.89	55.93	55.73	59.21	59.01	60.53	60.32	60.86	60.66	64.07	63.83	62.07	61.84	
18.75	46.81	46.69	55.77	55.56	59.05	58.84	60.37	60.16	60.70	60.50	63.91	63.66	61.89	61.66	
18.50	46.70	46.57	55.67	55.46	58.95	58.74	60.27	60.06	60.61	60.40	63.81	63.56	61.79	61.55	
18.25	46.61	46.48	55.56	55.35	58.84	58.63	60.16	59.94	60.50	60.29	63.69	63.45	61.64	61.40	
18.00	46.39	46.27	55.31	55.14	58.57	58.42	59.88	59.73	60.23	60.08	63.39	63.22	61.39	61.23	
17.75	46.22	46.11	55.13	54.98	58.39	58.26	59.69	59.57	60.04	59.92	63.20	63.06	61.21	61.07	
17.50	46.07	45.91	55.00	54.81	58.26	58.08	59.56	59.39	59.92	59.75	63.07	62.88	61.05	60.86	
17.25	45.76	45.61	54.74	54.57	58.02	57.86	59.33	59.17	59.69	59.54	62.84	62.66	60.73	60.58	
17.00	45.15	45.02	54.20	54.05	57.51	57.38	58.83	58.71	59.20	59.09	62.35	62.22	60.15	60.05	
16.75	44.68	44.76	53.75	53.76	57.10	57.10	58.43	58.43	58.82	58.82	61.97	61.94	59.84	59.88	
16.50	44.56	44.57	53.49	53.50	56.81	56.82	58.13	58.15	58.53	58.54	61.65	61.70	59.59	59.62	
16.25	44.42	44.42	53.31	53.31	56.63	56.63	57.95	57.95	58.35	58.35	61.46	61.46	59.44	59.44	
16.00	44.18	44.19	53.02	53.02	56.33	56.33	57.65	57.65	58.05	58.05	61.14	61.14	59.06	59.06	
15.75	43.86	43.86	52.51	52.51	55.77	55.77	57.08	57.08	57.49	57.49	60.50	60.50	58.29	58.29	
15.50	43.60	43.60	52.08	52.08	55.30	55.29	56.58	56.58	57.01	57.01	59.96	59.96	57.74	57.74	
15.25	43.34	43.34	51.71	51.71	54.90	54.90	56.18	56.18	56.62	56.62	59.53	59.53	57.34	57.34	
15.00	43.15	43.15	51.35	51.35	54.49	54.49	55.76	55.76	56.20	56.20	59.06	59.06	56.93	56.93	



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Cross-Section Location Map

SCALE	1° - 330
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Ber Length Co. Co One Inch. Adju	iginal Decreing Equals at Scale Accordingly

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Figure 2. Model Cross-Sections for Existing and Proposed Conditions.

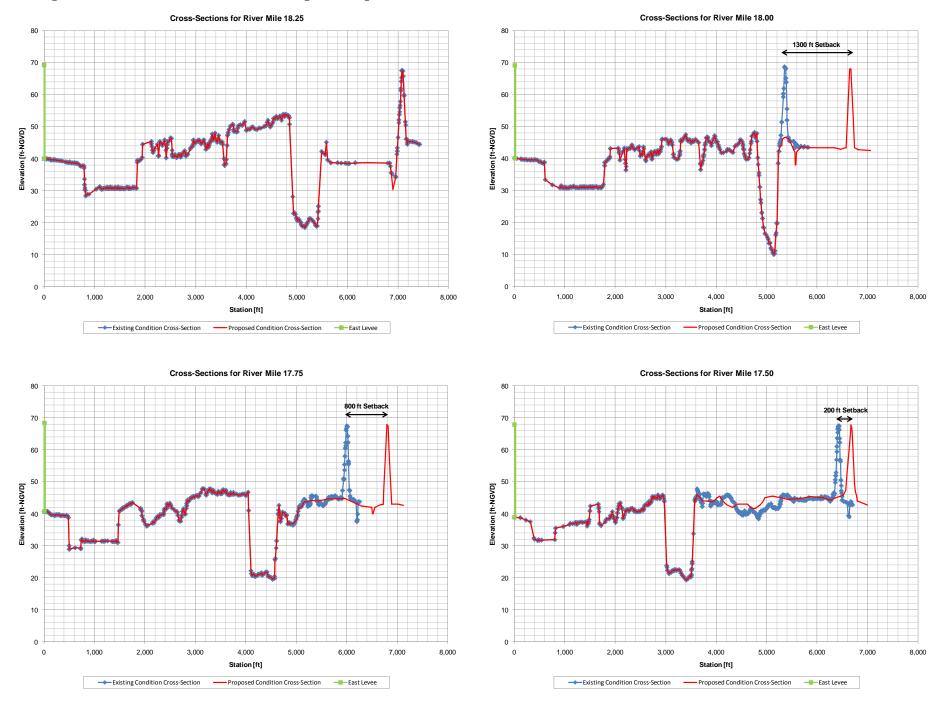


Figure 2 Continued

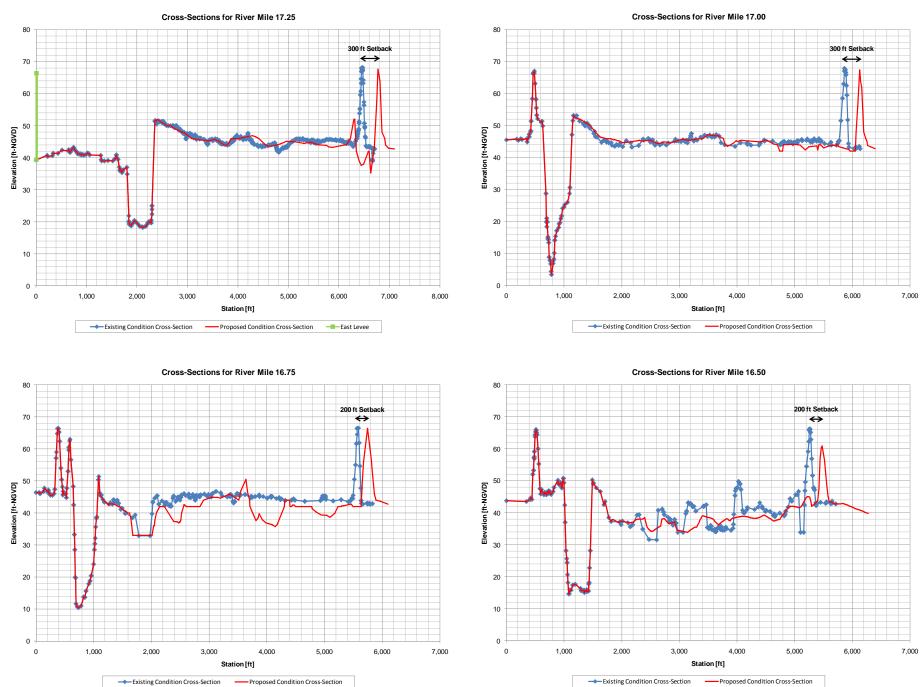


Figure 3:
1-in-100 AEP and 1-in-200 AEP Feather River Water Surface Profiles for Existing and Proposed Conditions (River Miles 15.0-23.0)

